

PREPARED BY



Model Input Status Report

H&H Modeling: Operations Model

Pensacola Hydroelectric Project Project No. 1494

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Executive Summary

Mead & Hunt is assisting Grand River Dam Authority (GRDA) with its intent to relicense the Pensacola Hydroelectric Project (Project), which is regulated by the Federal Energy Regulatory Commission (FERC). Flood control operations at the Project are regulated by the United States Army Corps of Engineers (USACE). GRDA proposed certain hydraulic and operations models for the relicensing study. The FERC's study plan determination requires GRDA to provide a model input status report. Mead & Hunt has performed this task on behalf of GRDA. This report documents the model inputs related to the Operations Model to be presented at the conference call.

USACE's RiverWare period-of-record model is a tool used by USACE Southwestern Division, Tulsa District (SWT) to simulate reservoir operations on the Arkansas River system upstream of the gage at Van Buren, Arkansas, including the Project. This model uses a daily time step and includes over 30 reservoirs.

Mead & Hunt is developing a Flood Routing Model for GRDA to replicate, as closely as possible, the Project flood routing decisions in the USACE RiverWare model as an input to the Operations Model. The Flood Routing Model is needed to investigate hypothetical design floods and alternative operating scenarios that would be difficult and time-consuming to program into the RiverWare model. The Flood Routing Model includes three reservoirs (Pensacola, Kerr, and Fort Gibson), which operate as a subsystem for flood routing, and uses daily time steps like the RiverWare model.

Mead & Hunt is developing an Operations Model for GRDA to simulate flood routing, hydropower scheduling, and other constraints on an hourly time step to support the Project relicensing effort. Because electricity prices vary widely within a day, hourly time steps provide improved accuracy for hydropower operations simulation. Output from the Flood Routing Model, most importantly the average daily total discharge, will be used as an input to the Operations Model. The Operations Model will seek to optimize the hydropower generation revenue at each facility while simultaneously satisfying various physical and operational constraints, including the flood routing decisions based on the RiverWare model as simulated in the Flood Routing Model. The Operations Model includes Pensacola Dam (Pensacola Hydroelectric Project) and Kerr Dam (Markham Ferry Hydroelectric Project), which is downstream of the Pensacola Dam. Both the Pensacola Dam and Kerr Dam are owned and operated by GRDA, and flood control operations at both projects are regulated by USACE.

The Flood Routing Model and Operations Model will be validated against the RiverWare model using the common metrics of the Coefficient of Determination (R^2) and the Nash-Sutcliffe Efficiency (NSE) to evaluate modeled total discharge and elevation.

List of Abbreviations and Terms

CADSWES	Center for Advanced Decision Support for Water and Environmental Systems
CFR	Code of Federal Regulations
CFS	Cubic Feet Per Second
CHM	Comprehensive Hydraulic Model
DO	Dissolved Oxygen
EEC	English Electric Company Limited
GRDA	Grand River Dam Authority
FERC	Federal Energy Regulatory Commission
ISR	Input Status Report
kW	kilowatt
MWh	Megawatt-hour
NAVD88	North American Vertical Datum of 1988
NGVD29	National Geodetic Vertical Datum of 1929
NSE	Nash-Sutcliffe Efficiency
OAC	Oklahoma Administrative Code
PD	Pensacola Datum
POR	Period of Record
Project	Pensacola Hydroelectric Project
PSP	Proposed Study Plan
R ²	Coefficient of Determination
RSP	Revised Study Plan
SPD	Study Plan Determination
SWT	Southwestern Division, Tulsa District
TW	Tailwater
USACE	United States Army Corps of Engineers
USGS	United States Geological Survey
VBA	Visual Basic for Applications

1. Introduction and Background

1.1 Project Description

The Pensacola Hydroelectric Project is owned and operated by GRDA and regulated by the FERC. The Pensacola Dam is in Mayes County, Oklahoma on the Grand-Neosho River. Pensacola Dam impounds Grand Lake. Construction of Pensacola Dam was completed in 1940. Authorized purposes for the Project include flood control, recreation, and hydropower. **Figure 1** displays the study area. Downstream of Pensacola Dam, GRDA also owns and operates the Robert S. Kerr Dam as the Markham Ferry Hydroelectric Project. Kerr Dam is also in Mayes County and impounds Lake Hudson, also known as Markham Ferry Reservoir. Flood control operations at both Pensacola Dam and Kerr Dam are regulated by USACE.

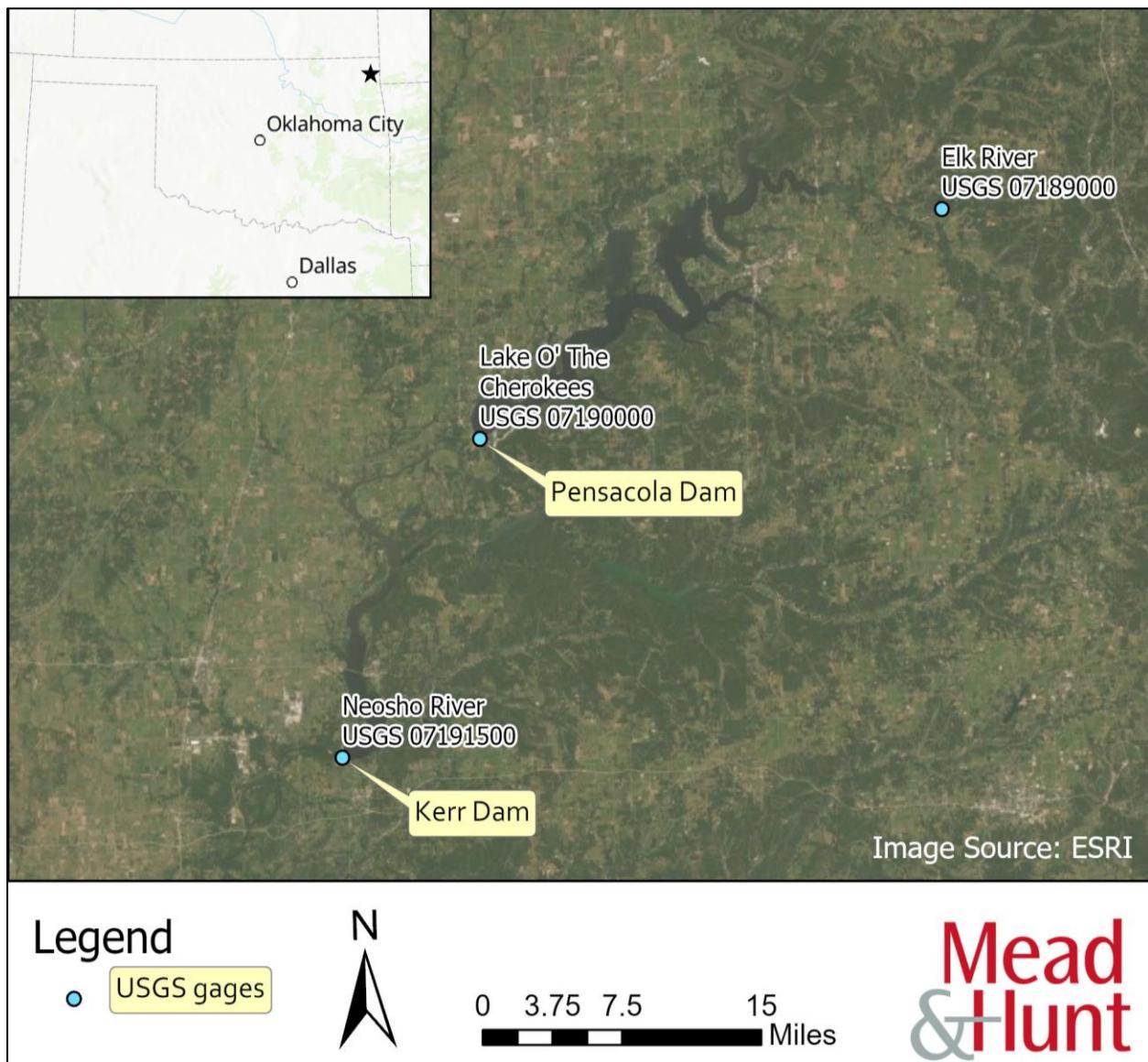


FIGURE 1. OPERATIONS MODEL STUDY AREA

1.2 Study Plan Proposals and Determination

GRDA is currently relicensing the Project. The timeline of study plan proposals and determination is as follows:

1. On April 27, 2018, GRDA filed its Proposed Study Plan (PSP) to address hydrologic and hydraulic modeling in support of its intent to relicense the Project.
2. On September 24, 2018, GRDA filed its Revised Study Plan (RSP).
3. On November 8, 2018, the FERC issued its Study Plan Determination (SPD) for the Project.
4. On January 23, 2020, the FERC issued an Order on the Request for Clarification and Rehearing, which clarified the timeline for certain milestones applicable to the relicensing study plan.

The PSP and RSP recommended the development of an Operations Model to synthesize and create events that inform or set boundaries for the Comprehensive Hydraulic Model (CHM). The FERC's SPD included the following determination specific to the Operations Model:

We recommend that GRDA demonstrate in the ISR [Initial Study Results report, scheduled for September 2021] that it has validated its model results against the RiverWare output.

This report provides an input status and documents the proposed development of the Flood Routing Model and Operations Model, as well as the proposed validation of results against the USACE RiverWare output.

2. USACE RiverWare Model

The United States Army Corps of Engineers RiverWare period-of-record model is a tool used by USACE Southwestern Division, Tulsa District to simulate reservoir operations on the Arkansas River system upstream of the gage at Van Buren, Arkansas (USACE, 2020). The model simulates hydrologic inflows, evaporation, seepage, water deliveries, reservoir management, flood control, and hydropower production on a daily time step from 1940 through 2017. The model area includes more than 30 reservoirs, and the main control point for flood routing decisions is at the Van Buren gage. When flows at Van Buren are projected to exceed the seasonal guide curve, upstream reservoirs store water to limit flow at Van Buren. Other reservoirs or reservoir subsystems also have their own flood release restrictions. Reservoir balance levels throughout the system are managed to limit flooding systemwide.

Under Section 7 of the Flood Control Act of 1944 (CFR, 1944), the USACE has the responsibility to prescribe releases from Pensacola Dam and Kerr Dam under active or anticipated flood conditions (CFR, 1945). The USACE may exercise direct control over the facilities or provide instructions to GRDA to manage releases for the purpose of basin-wide flood mitigation. The RiverWare model illustrates how reservoir levels at Pensacola Dam and Kerr Dam may be increased during a large flood event impacting Van Buren, not because the spillway capacity or downstream channel capacity is exceeded at either facility, but because water is held back to limit flow at Van Buren.

USACE SWT provided Mead & Hunt with time series, tabular, and other data from the RiverWare model, and examples of these data are included in **Appendix A**. The model domain is shown in **FIGURE 2**.

Mead & Hunt downloaded the RiverWare Technical Documentation from the Center for Advanced Decision Support for Water and Environmental Systems (CADSWES) at University of Colorado Boulder, College of Engineering and Applied Science website and referenced it to understand how to replicate the modeling methods applicable to this study (CADSWES, 2020). CADSWES develops RiverWare, a river and reservoir/hydropower planning and management tool that is licensed by the University of Colorado Technology Transfer Office, and widely used by agencies and consultants. Documentation for the related TAPER model was also provided by USACE (Steffen, Stringer, Daylor, Neumann, & Zagona, 2015). USACE SWT and CADSWES staff also provided aid in understanding the RiverWare model and how to apply its objects and methods to this study.

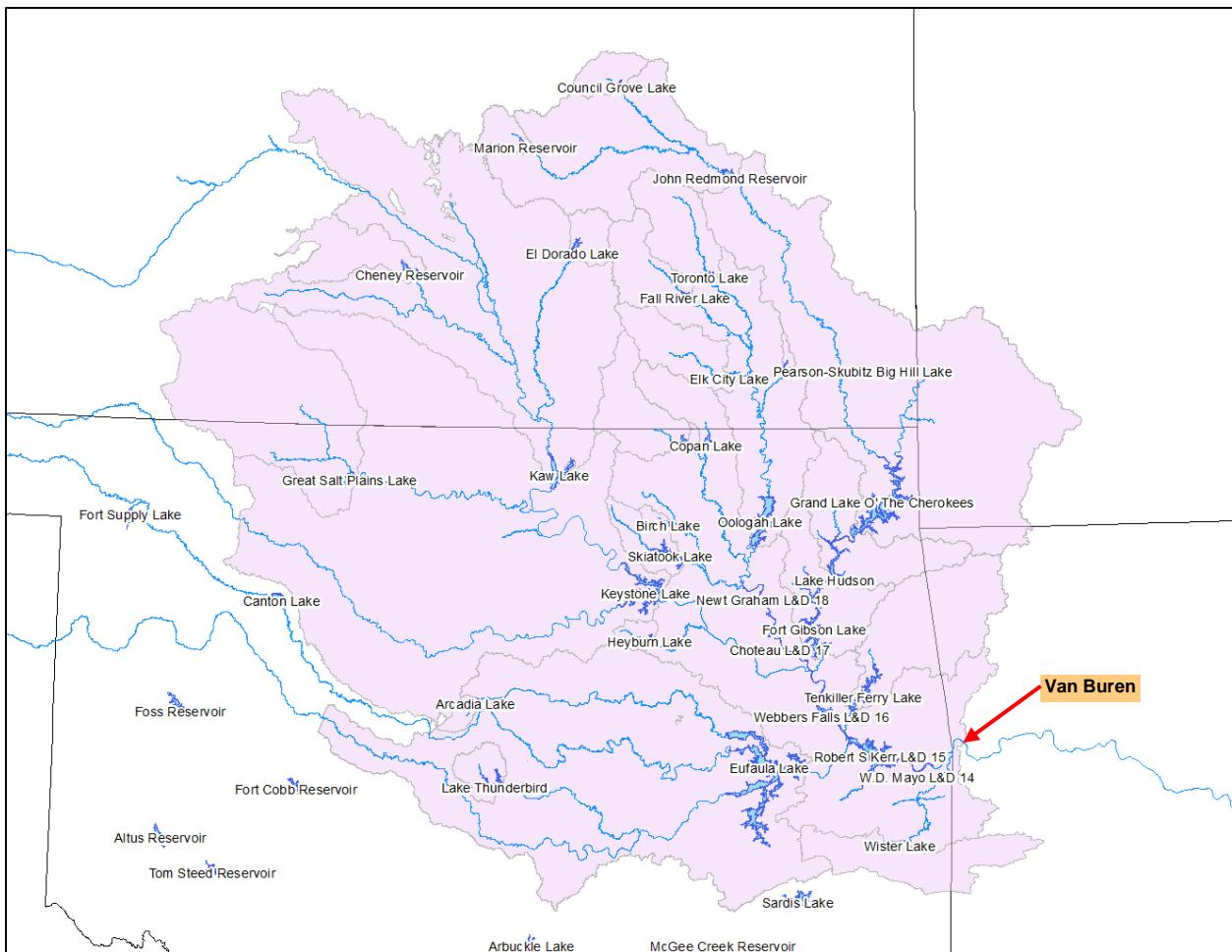


FIGURE 2. RIVERWARE MODEL DOMAIN

3. Flood Routing Model

The FERC SPD recommends GRDA demonstrate it has validated its model results against the RiverWare model output. Mead & Hunt is developing a Flood Routing Model to replicate, as closely as possible, the Project flood routing decisions in the RiverWare model as an input to the Operations Model. The Flood Routing Model includes (from upstream to downstream) Pensacola Dam, Kerr Dam, and Fort Gibson Dam because these operate as a subsystem for reservoir level balancing. The Flood Routing Model is needed to investigate hypothetical design floods and alternative operating scenarios that would be difficult and time-consuming to program into the RiverWare model.

3.1 Input Data

Input data for the Flood Routing Model includes river discharge upstream of Pensacola Dam, local inflows to each reservoir, evaporation and seepage rates, reservoir stage-storage-area tables, reservoir operating level tables, maximum regulated spill tables, induced surcharge tables, seasonal Project target reservoir elevation table, and hydrologic routing coefficient tables from the RiverWare model.

3.2 Proposed Methodology

The Flood Routing Model is being developed using Visual Basic for Applications (VBA) code within an Excel spreadsheet. The VBA code loops through the time series calculations to solve the model. The VBA code computes formulas to calculate the system state and flood routing decisions for each daily time step, then copies the formulas to the next daily time step and stores the results as plain text in the Excel spreadsheet. In this way, the file size is reduced and the computation speed improved for the model.

Minimum surcharge and maximum regulated outflows are calculated individually from upstream to downstream. Operating levels are balanced upstream to downstream and highest to lowest, within calculated outflow limits. Individual dam and Lower Grand (Neosho) subsystem limits are checked. Conservation pool rules and hydropower rules are ignored for purposes of flood routing. Modeled objects include reservoirs, control points, and reaches.

Methods from the RiverWare model replicated in the Flood Routing Model include:

- Flow combined at nodes and routed downstream hydrologically.
- Evaporation calculated using historical evaporation rates considering modeled reservoir area.
- Constant seepage assumed: 10 cubic feet per second (cfs) at Kerr Dam and 20 cfs at Fort Gibson Dam.
- Minimum surcharge and maximum regulated outflow calculated using the USACE SWT flat top surcharge method.
- Allowable rising release change and allowable falling release change limit how quickly the controlled releases are increased or decreased, subject to other limitations such as minimum surcharge.
- Operating level balancing seeks to maintain similar balance levels in each reservoir.
- Regulating discharges of 100,000 cfs established in the water control manual are considered for Pensacola Dam, Kerr Dam, and Fort Gibson Dam (USACE, 1980).

4. Operations Model

Mead & Hunt is developing an Operations Model to simulate flood routing, hydropower scheduling, and other Project constraints on an hourly time step. Because electricity prices vary widely within a day, hourly time steps provide improved accuracy for hydropower operations simulation. The Operations Model is being developed using VBA code within an Excel spreadsheet, as described in more detail below.

4.1 Vertical Datums

Data sources for this study use a variety of vertical datums. Unless otherwise noted, data related to Pensacola Dam and Grand Lake is referenced to the Pensacola Datum (PD) and data related to Kerr Dam and Lake Hudson is referenced to the National Geodetic Vertical Datum of 1929 (NGVD29). To convert from PD to NGVD29, add 1.07 feet. To convert from NGVD29 to the North American Vertical Datum of 1988 (NAVD88), add 0.33 feet. **Figure 3** displays datum transformations and conversions (Hunter, Trevisan, Villa, & Smith, 2020).

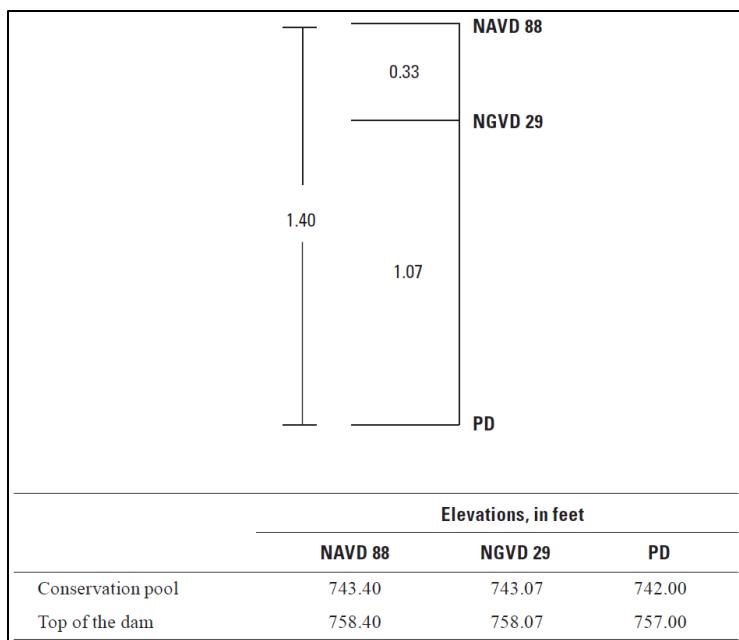


FIGURE 3. DATUM TRANSFORMATIONS AND CONVERSIONS

SOURCE: (HUNTER, TREVISAN, VILLA, & SMITH, 2020).

4.2 Input Data and Preparation

Mead & Hunt obtained data for the Operations Model from GRDA and USACE. Input data includes project drawings, rating curves, time series data, and information from the RiverWare model. Most of the time series data was available in hourly or sub-hourly time steps from April 1, 2004 through December 31, 2017 (the end date in the RiverWare model), so this period was selected for analysis. Descriptions and sources of each input data set are described below, along with descriptions of the methods used to prepare the data for input to the Operations Model. **Appendix B** contains a collection of the information described in this section.

4.2.1 Headloss vs. Turbine Discharge

GRDA sent Mead & Hunt drawings of the Pensacola and Kerr hydroelectric facilities showing the dimensions of the water passages from the intake to the tailrace at each powerhouse (GRDA, 1961), (GRDA, 1987). The dimensions shown on these drawings were used to estimate the hydraulic friction and form losses as part of the turbine net head calculation.

Facility drawings for the Pensacola and Kerr facilities were used to identify specific friction loss (i.e., conduit roughness) and minor loss (e.g., bends, contractions, or entrance and exit losses) components between the reservoirs and the turbines. It is common practice for turbine manufacturers to include the hydraulic losses from the scroll case entrance through the tailrace in the turbine hill curve efficiencies, so these losses were specifically excluded from the headloss calculations.

Total headloss was calculated using a combination of the Darcy-Weisbach formula for friction losses and assumed minor loss coefficients. To simplify spreadsheet calculations, major and minor losses are expressed in terms of a flow-based headloss coefficient, Kq, according to the equations below:

$$H_L = Kq * Q^2$$

$$Kq = \frac{0.025 * f * L}{D_h^5}$$

$$Kq = \frac{Kv}{2gA^2}$$

where:

H_L = headloss (ft)

Kq = flow-based headloss coefficient

Kv = velocity-based headloss coefficient

Q = turbine discharge (ft³/s)

L = pipe length (ft)

f = Darcy-Weisbach resistance coefficient

D_h = Hydraulic diameter (ft)

A = pipe wetted area (ft²)

g = gravity constant (32.16 ft/s²)

The resistance coefficient varies with pipe size, pipe roughness, fluid kinematic viscosity, and discharge. Resistance coefficients were determined based on the following equations, which approximate the curves of the Moody Diagram.

Von Karman-Prandtl equation for smooth pipe flow:

$$\frac{1}{\sqrt{f}} = 2 \log_{10}(Re\sqrt{f}) - 0.8$$

$$Re = D_h V \nu$$

where:

Re = Reynold's Number

V = average velocity (ft/s)

ν = kinematic viscosity of fluid (ft²/s)

Von Karman-Prandtl equation for rough pipe flow:

$$\frac{1}{\sqrt{f}} = 2 \log_{10} \frac{D_h}{2k} + 1.74$$

where:

k = absolute roughness of pipe wall (ft)

Colebrook-White equation for transition flow:

$$\frac{1}{\sqrt{f}} = -2 \log_{10} \left(\frac{k}{3.7D_h} + \frac{2.51}{Re\sqrt{f}} \right)$$

Friction losses for Pensacola were calculated assuming water at 50° F, roughness of 0.00015 feet (steel), and acceleration due to gravity of 32.146 ft/s². Minor loss coefficients were selected using the appropriate Hydraulic Design Criteria charts (USACE, 1987).

The total calculated flow-based headloss coefficient (Kq) for Pensacola was 4.50×10^{-7} , and for Kerr was 4.11×10^{-9} .

4.2.2 Reservoir Elevation-Storage-Area Curves

For model validation purposes, reservoir elevation-storage-area rating tables for Pensacola and Kerr from the RiverWare model will be used in the Operations Model. Other more recent bathymetric survey data may be incorporated post-validation to improve later simulations (Dewberry, 2011), (Hunter, Trevisan, Villa, & Smith, 2020).

4.2.3 Turbine Efficiency

Pensacola

Efficiency curves for the Pensacola generators were transcribed from the Siemens efficiency calculations (Siemens, 1999) in terms of generator efficiency vs. percent rated generator load, assuming rated brake power of 18,112 kilowatt (kW) at 0.90 power factor.

Efficiency vs. turbine discharge vs. net head curves for the Pensacola turbines (air valves closed condition) were transcribed from the Voith hill curve (Voith, 1997). Efficiency curves for the condition when the dissolved oxygen (DO) air valves are fully open were transcribed from the Accusonic Unit 3 preliminary performance test report (Walsh, 1999).

Two efficiency curves were developed for Pensacola: one for normal operation with all the DO air valves closed, and one for derated operation with all the DO air valves fully open (90 degrees open).

The normal operation turbine efficiency vs. turbine discharge vs. net head curve was transcribed from the Voith hill curve into a Excel spreadsheet table. The generator efficiency vs. load curve was then multiplied by the turbine efficiencies to develop a total turbine-generator efficiency vs. discharge vs. net head table.

The derated operation (air valves open) turbine efficiencies at a net head of 117.5 feet were transcribed from the Accusonic report. The ratio of derated turbine efficiency to normal turbine efficiency for a given turbine discharge was assumed to be constant across the entire range of net head values used (95 feet to 140 feet). After extrapolating the derated turbine efficiencies, the generator efficiency vs. load curve was again applied to develop a total turbine-generator efficiency vs. turbine discharge vs. net head table for derated operation.

Kerr

Efficiency curves for the Kerr generators were transcribed from the table in Section 2.3.1 of the Alstom generator calculations (Bertrand, 2009) in terms of generator efficiency vs. percent rated generator load, assuming rated brake power of 33,036 kW at 0.93 power factor.

Power vs. turbine discharge vs. net head curves for the Kerr turbines (pre-refurbishment) were transcribed from the original 1962 prototype unit curves (EEC, 1962a). Relative turbine efficiencies for the Kerr turbines (post-refurbishment) were transcribed from the Alstom index test results (Alstom, 2010).

The efficiency curves for Kerr were developed based on the original 1962 prototype efficiency curve and the 2010 post-refurbishment index testing. The total turbine-generator efficiency vs. turbine discharge vs. net head table for the original units was developed using a similar methodology as for Pensacola. Alstom reported index test results as a relative turbine efficiency (refurbished units relative to the original 1962 prototype units) vs. refurbished turbine power at a net head of 56 feet. Index efficiencies were applied across the entire domain of the original hill curve by multiplying the relative efficiency at an equivalent discharge by the maximum original turbine efficiency value at each net head.

4.2.4 Turbine Maximum Discharge

The maximum allowable turbine discharge (to avoid cavitation) vs. net head limit for Pensacola was determined from the entrance edge cavitation lines on the Voith hill curve (Voith, 1997). The maximum turbine discharge vs. net head limit for Kerr was determined from the limit of Kaplan operation line on the original 1962 prototype unit curves (EEC, 1962b).

4.2.5 Tailwater Rating Curves

The tailwater (TW) elevation vs. total discharge rating curve for Pensacola Dam was developed based on the historical observed TW elevation and discharge paired data provided by GRDA. The elevation-discharge data pairs were plotted and a table of paired values was developed based on a best fit through the observed points.

The TW elevation vs. total discharge rating curve for Kerr Dam was developed using United States Geological Survey (USGS) gage data: Neosho River near Langley, OK (USGS Gage No. 07190500). The gage data pairs were plotted and a table of paired values was developed based on a best fit through the gage data points.

4.2.6 Unit Outages

Individual turbine unit outages are recorded in terms of the start and end date/time of each outage. The outages can be either scheduled or unscheduled, and the type of outage can be forced, planned, maintenance, or de-rated generation output (e.g., DO air valve operation at Pensacola).

Records of the date and time for the beginning and ending of outages for individual turbine-generator units were used to construct time series of unit outages. Overlapping outages were reconciled when the beginning or ending timestamp of a listed outage fell within the timeframe of another outage for the same unit. An hourly time series was constructed to indicate the status of each individual unit (online or offline) for each time step. Lastly, the total number of units online for each time step at each facility was determined. This data was used by the operations model to determine the total turbine discharge capacity available for a given time step.

4.2.7 Dissolved Oxygen Air Valves

Dissolved Oxygen data, including measured DO levels and the status of each of the six Pensacola DO air valves, was available between January 1, 2004 and April 13, 2006 (non-contiguous data). Oklahoma Water Resources Board Title 785.45-5-12 describes seasonal DO concentrations needed to support various subcategories of Fish and Wildlife beneficial use designations for streams (OAC, 2013). The prescribed seasonal DO concentrations were considered along with records of measured DO concentrations and individual air valve status from Pensacola to estimate when the air valves may have typically been opened during the modeled period of analysis, as described below. The model accounts for the status of the air valves by switching between two different turbine efficiency curves to represent the decrease in turbine efficiency when the air valves are open.

For Pensacola, the prescribed seasonal DO concentrations were considered along with records of DO levels; individual air valve status; actual generation output; and head, efficiency, and flow data to estimate when the air valves may have typically been opened during the modeled period of analysis. There are no DO air valves at Kerr.

4.2.8 Electricity Prices

Hourly day-ahead and real-time locational marginal prices (in terms of dollars per megawatt-hour [MWh]) for electrical energy produced at Pensacola and Kerr were available from March 1, 2014 to December 31, 2020.

Daily natural gas settlement prices for the Henry Hub trading point were available from March 1, 2004 through December 31, 2020. Natural gas prices were used to index the electricity prices across the full range of analysis dates.

Hourly electricity prices for both day-ahead and real-time markets over the period between March 1, 2014 and December 31, 2020 were divided by the corresponding daily natural gas price to determine the hourly heat rate pattern for this period. This heat rate pattern was then applied to corresponding dates when no hourly electricity prices were available (prior to March 1, 2014). The assumed hourly heat rate was then multiplied by the actual daily historical natural gas settlement prices going back to April 1, 2004 to estimate the historical electricity prices for the period when actual hourly data was not available.

4.2.9 Spillway Capacity

USACE spillway discharge capacity ratings were reviewed for Pensacola and Kerr, but for the purposes of model validation the rating tables from the RiverWare model will be used instead (USACE, 1990), (USACE, 1991). Other more detailed spillway capacity data may be incorporated post-validation to improve later simulations.

4.3 Proposed Methodology

The objective function of the Operations Model is to maximize the hydropower generation revenue at each facility while simultaneously satisfying various physical and operational constraints (e.g., reservoir level management, flood routing from the Flood Routing Model, scheduling of power sales, and operation of the turbines within their allowable range). Many dependent functions comprise the overall objective function and are summarized here:

- Total discharge computed by the Flood Routing Model is also used by the Operations Model when the reservoir transitions into flood operations. Otherwise, for normal operations, discharge is determined based on optimal hydropower generation scheduling.
- Modeled revenue is a function of scheduled power and electricity price (both day-ahead and real-time).
- Power is a function of turbine discharge, net head, and total turbine-generator efficiency.
- Net head is a function of headwater, tailwater, and headloss.
- Efficiency is a function of turbine discharge, net head, and DO air valve open/closed status (Pensacola only).

- Turbine discharge is a function of best efficiency point discharge, maximum allowable discharge (avoiding possible cavitation), current reservoir storage volume, forecast inflow volume, electricity price, production cost, the number of units online.
- Reservoir storage volume is a function of inflow, turbine discharge, spillway discharge, evaporation, and seepage (Kerr only, no seepage at Pensacola in RiverWare).
- Other parameter dependencies have been discussed in the description of the input data preparation in **Section 4.2**.

The Operations Model is driven by VBA code to do two things: 1) solve an iterative loop for the net head values, and 2) step through the time series one day at a time. The Excel spreadsheets contain formulas to optimize the hydroelectric operations for a 24-hour period. The VBA code copies the formulas down from one day to the next as each 24-hour period is solved, preserving the solution values. In this way, the Excel spreadsheets are kept to a manageable size and overall calculation speed is improved.

At the beginning of the solution for a given 24-hour period, an estimated value for net head at each hourly time step is assumed (final values from one day are copied down to the next day to provide the first estimated value). The net head is used to calculate various parameters related to the turbine and spillway discharge, which in turn are used to calculate the TW elevation and headloss, which are then used to calculate the next estimated value for net head.

As each 24-hour period is solved, the VBA code copies the formulas down to the next day, preserving the values only (not the formulas) for the day that was just solved to reduce file size.

Each scenario model is solved first for Pensacola. The resulting total discharge (turbines plus spillway) from Pensacola is then hydrologically routed downstream to Kerr Dam in a manner consistent with the RiverWare hydrologic routing. The routed Pensacola Dam discharge for the given scenario, plus tributary inflows to Lake Hudson, are then combined and copied as inflow to the operations model for Kerr.

Hourly time series data produced in the operations model includes reservoir elevation, turbine discharge, spillway discharge, net head, power (scheduled day-ahead, real-time buy-back, real-time scheduled, and total), and revenue (day-ahead sales, buy-back cost, real-time sales, and total).

5. Validation

Model variables that will be used to validate performance of the Operations Model and Flood Routing Model against the RiverWare model output include total discharge from a reservoir and elevation of a reservoir. Reservoir storage and balance level are also available as validation parameters, but are simple corollaries for elevation. Because elevation is a more intuitive parameter for understanding the system state, it is proposed rather than the other corresponding parameters.

The date range for Operations Model validation will be April 1, 2004 to December 31, 2017. This is the overlapping date range for which data is available from the RiverWare model and when hourly data is available for the Operations Model. The Flood Routing Model may be validated against the RiverWare model using results as far back as 1940.

Performance metrics will include the Coefficient of Determination (R^2) and the Nash-Sutcliffe Efficiency (NSE). R^2 is an index of the degree of linear relationship between source and simulated data. R^2 predicts correlation between models and dispersion of data relative to that correlation. It does not evaluate accuracy, only correlation. NSE is an index of how well the source versus simulated data fits a perfect 1:1 correlation slope line. Plotting on a 1:1 line indicates consistent prediction at lower and higher values. However, NSE is more sensitive to extreme values. Formulas for R^2 and NSE are available in literature and given below for reference. The optimal value for both metrics is 1. For R^2 , the optimal trendline intercept is 0,0 and the optimal trendline slope is 1. **Table 1** lists qualitative ranges for evaluating model validation using these metrics (Moriasi, Gitau, Pai, & Daggupati, 2015).

$$R^2 = \left[\frac{\sum_{i=1}^n (O_i - \bar{O})(P_i - \bar{P})}{\sqrt{\sum_{i=1}^n (O_i - \bar{O})^2} \sqrt{\sum_{i=1}^n (P_i - \bar{P})^2}} \right]^2$$

NSE

$$1 - \frac{\sum_{i=1}^n (O_i - P_i)^2}{\sum_{i=1}^n (O_i - \bar{O})^2}$$

TABLE 1. SUMMARY OF PERFORMANCE METRICS

Metric	Range	Not Satisfactory	Satisfactory	Good	Very Good
R^2	0 to 1	≤ 0.60	0.60 to ≤ 0.75	0.75 to ≤ 0.85	> 0.85
NSE	$-\infty$ to 1	≤ 0.50	0.50 to ≤ 0.70	0.70 to ≤ 0.80	> 0.80

Validation of the Operations Model against the RiverWare model output will be performed using similar inputs as the RiverWare model for historical discharge, evaporation, seepage, stage-storage-area tables, reservoir operating level tables, maximum regulated spill tables, induced surcharge tables, seasonal Project target reservoir elevation table, and hydrologic routing coefficient tables. Following validation, some or all of these inputs and the associated flood routing methods may be updated and expanded to meet the needs of the study.

6. Summary

Mead & Hunt is developing a three reservoir version of the RiverWare model, referred to as the Flood Routing Model, to investigate operating alternatives and hypothetical (non-historical) flood events. The Flood Routing Model includes Pensacola, Kerr, and Fort Gibson Dams. Mead & Hunt is also developing an Operations Model to compute hourly hydropower scheduling, generation, and revenue at Pensacola and Kerr. The FERC SPD recommends GRDA demonstrate it has validated its model results against the RiverWare model output. The validation will compare the Flood Routing Model and Operations Model results against the RiverWare model output using R^2 and NSE correlation metrics. This report documents the development of the input data for the Operations Model that will be presented at the FERC-recommended conference call.

7. References

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Appendix A. USACE RiverWare Data

Pensacola

Elevation ft PD	Area acre	Storage acre-ft	Elevation ft PD	OPEVEL 1 - 16.1	Elevation ft PD	Max Regulated Spill cfs	Induced Surcharge cfs
696	12490	305932					
697	12813	318584					
698	13142	331561					
699	13487	344873					
700	13934	358558					
701	14259	372661					
702	14550	387069					
703	14846	401769					
704	15157	416769					
705	15584	432106					
706	15921	447863					
707	16263	463955					
708	16629	480402					
709	17016	497224					
710	17556	514468					
711	18036	532267					
712	18520	550554					
713	18986	569302					
714	19509	588551					
715	20190	608359					
716	20726	628825					
717	21262	649820					
718	21830	671364					
719	22478	693521					
720	23244	716333					
721	23859	739896					
722	24443	764054					
723	25007	788782					
724	25592	814081					
725	26354	840006					
726	26991	866684					
727	27655	894007					
728	28359	922021					
729	29035	950728					
730	29745	980096					
731	30386	1010168					
732	31052	1040888					
733	31788	1072304					
734	32607	1104503					
735	33530	1137530					
736	34275	1171445					
737	34989	1206086					
738	35760	1241453					
739	36638	1277662					
740	37788	1314761					
741	38918	1353101					
742	40021	1392678					
743	40636	1433025					
744	41221	1473920					
745	41779	1515412					
746	43551	1558073					
747	45323	1602507					
748	47095	1648714					
749	48867	1696692					
750	50639	1746443					
751	52411	1797965					
752	54184	1851261					
753	55956	1906328					
754	57728	1963167					
755	59300	2021679					
756	61100	2081877					
757	62950	2143900					
758	64800	2207773					

Hudson

Elevation ft M.S.L.	Area acre	Storage acre-ft	Elevation ft M.S.L.	OPEVEL 1 - 16.1	Elevation ft M.S.L.	Max Regulated Spill cfs	Induced Surcharge cfs
641	22375	547111					
642	23120	569858					
643	23870	593352					
644	24620	617596					
645	25400	642605					
648	27910	722482					

Parsons-Commerce

Routing Coefficients	
Lag Coeff	
1	0.353
2	0.4568
3	0.1344
4	0.0395
5	0.0116
6	0.0034
7	0.001
8	0.0003

Commerce-Pensacola

Routing Coefficients	
Lag Coeff	
1	0.6
2	0.4

Pensacola-Hudson

Routing Coefficients	
Lag Coeff	
1	1
2	1

Hudson-Ft Gibson

Routing Coefficients	
Lag Coeff	
1	1
2	1

Appendix B. Operations Model Input Data

HEAD LOSS CALCULATIONS - 1 OF 6

Component	Pipe Size (ft)	Description	Reference for Headloss Estimation	Minor Losses			Total Headloss	
				Kv (ft ²)	A (ft)	L (cfs)	Kq (ft)	Q (cfs)
Trashrack	---	Trashrack losses neglected: very large area, low velocity	---	---	---	---	---	---
Inlet	15x15	Standard sharp-edged inlet loss	---	0.50	225.00	---	1.54E-07	2505 0.96
Contraction: 15' sq. to 15' diam.	15	Area 1 = 225.0 ft ² ; Area 2 = 176.7 ft ² ; A2/A1 = 0.78	[USACE HDC 228-4]	0.07	176.71	---	3.49E-08	2505 0.22
42° bend	15	r = 75'; Assume r/D=5	[USACE HDC 228-1]	0.075	176.71	---	3.74E-08	2505 0.23
Butterfly Valve	15	15' Diameter Butterfly Valve	[USACE HDC 331-1]	0.34	176.71	---	1.68E-07	2505 1.06
Penstock Friction	15	194' +/- Total Length	Darcy-Weisbach	---	---	194	5.58E-08	2505 0.35
				Totals: 4.50E-07 2505 2.82				

Governing Equations:

$$H = Q^2 / (C^2 * A^2 * 2g) \quad \text{where:}$$

$$Q = C * A * (2gH)^{1/2}$$

$$A = B * Go$$

$$H = Kv * V^2 / (2g)$$

$$Q = A * (2gH/Kv)^{1/2}$$

$$Kv = 1/C^2$$

$$H = Q^2 * Kq$$

$$Q = (H/Kq)^{1/2}$$

$$Kq = 1/(C^2 * A^2 * 2g)$$

g (gravity constant) = 32.146 ft/s² [HDC 000-1]

H = energy head immediately upstream of gate, measured to centerline

Q = gate discharge (cfs)

A = open area of gate (ft²)

B = gate width (feet)

Go = gate vertical opening (feet)

V = velocity through open area of gate (ft/s)

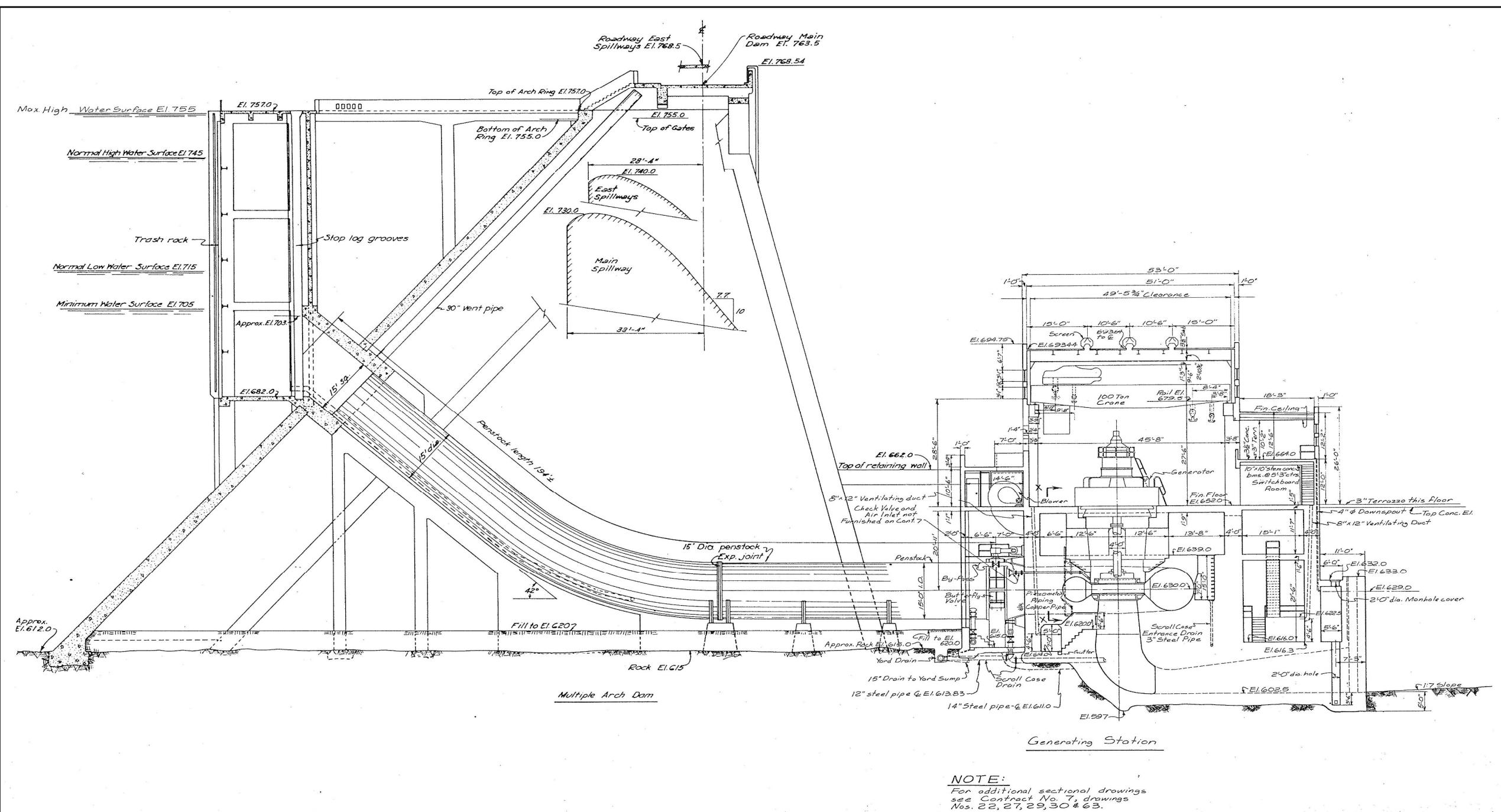
C = discharge coefficient

Kv = gate valve loss coefficient (velocity-based)

Kq = gate valve loss coefficient (discharge-based)

Site Latitude = 36.5 deg

Site Elevation = 750 feet

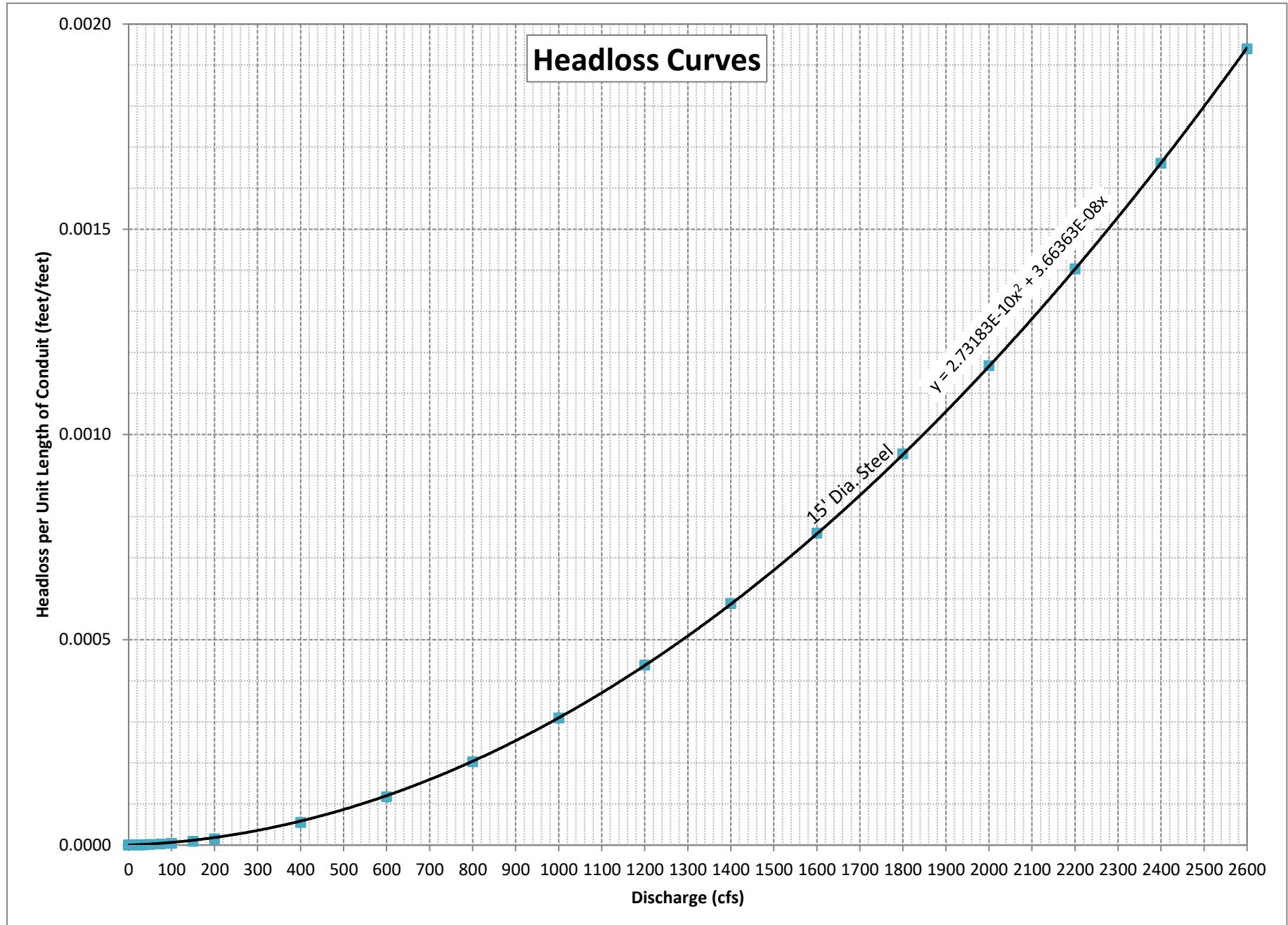


HEAD LOSS CALCULATIONS - 2 OF 6

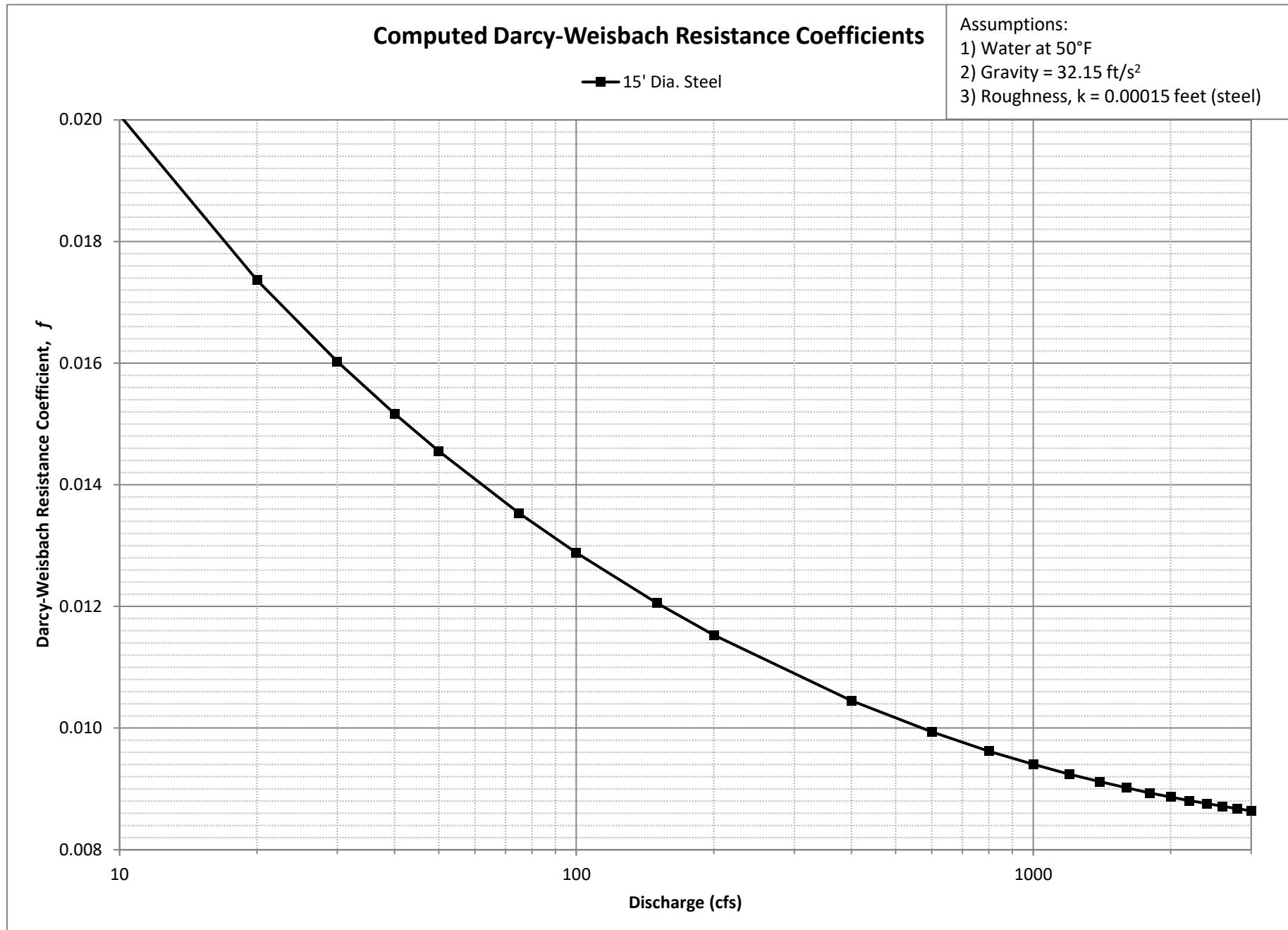
Darcy-Weisbach Resistance Coefficients for Various Pipe Sizes and Discharges

Discharge (cfs)	Conduit Diameter (feet)	Gravity Constant	Wetted Area (ft ²)	Wetted Perimeter (feet)	Hydraulic Diameter (feet)	Velocity (ft/sec)	Fluid Type	Kinematic Viscosity ft ² /s		Specific Weight	Reynolds Number	Conduit Material	Relative Roughness (feet)	Laminar Zone (f)			
								3	2					Conduit Material	Relative Roughness (feet)	Laminar Zone (f)	Critical Zone (f)
1	15	32.14577	176.71	47.12	15	0.0	Water, 50° F	1.41E-05	1.41E-05	1.94	62.36	6.03E+03	Steel or wrought Iron	0.00015	1.00E-05	0.010609	indeterminate
2	15	32.14577	176.71	47.12	15	0.0		1.41E-05	1.41E-05	1.94	62.36	1.21E+04		0.00015	1.00E-05	0.005304	indeterminate
3	15	32.14577	176.71	47.12	15	0.0		1.41E-05	1.41E-05	1.94	62.36	1.81E+04		0.00015	1.00E-05	0.003536	indeterminate
10	15	32.14577	176.71	47.12	15	0.1		1.41E-05	1.41E-05	1.94	62.36	6.03E+04		0.00015	1.00E-05	0.001061	indeterminate
20	15	32.14577	176.71	47.12	15	0.1		1.41E-05	1.41E-05	1.94	62.36	1.21E+05		0.00015	1.00E-05	0.000530	indeterminate
30	15	32.14577	176.71	47.12	15	0.2		1.41E-05	1.41E-05	1.94	62.36	1.81E+05		0.00015	1.00E-05	0.000354	indeterminate
40	15	32.14577	176.71	47.12	15	0.2		1.41E-05	1.41E-05	1.94	62.36	2.41E+05		0.00015	1.00E-05	0.000265	indeterminate
50	15	32.14577	176.71	47.12	15	0.3		1.41E-05	1.41E-05	1.94	62.36	3.02E+05		0.00015	1.00E-05	0.000212	indeterminate
75	15	32.14577	176.71	47.12	15	0.4		1.41E-05	1.41E-05	1.94	62.36	4.52E+05		0.00015	1.00E-05	0.000141	indeterminate
100	15	32.14577	176.71	47.12	15	0.6		1.41E-05	1.41E-05	1.94	62.36	6.03E+05		0.00015	1.00E-05	0.000106	indeterminate
150	15	32.14577	176.71	47.12	15	0.8		1.41E-05	1.41E-05	1.94	62.36	9.05E+05		0.00015	1.00E-05	0.000071	indeterminate
200	15	32.14577	176.71	47.12	15	1.1		1.41E-05	1.41E-05	1.94	62.36	1.21E+06		0.00015	1.00E-05	0.000053	indeterminate
400	15	32.14577	176.71	47.12	15	2.3		1.41E-05	1.41E-05	1.94	62.36	2.41E+06		0.00015	1.00E-05	0.000027	indeterminate
600	15	32.14577	176.71	47.12	15	3.4		1.41E-05	1.41E-05	1.94	62.36	3.62E+06		0.00015	1.00E-05	0.000018	indeterminate
800	15	32.14577	176.71	47.12	15	4.5		1.41E-05	1.41E-05	1.94	62.36	4.83E+06		0.00015	1.00E-05	0.000013	indeterminate
1000	15	32.14577	176.71	47.12	15	5.7		1.41E-05	1.41E-05	1.94	62.36	6.03E+06		0.00015	1.00E-05	0.000011	indeterminate
1200	15	32.14577	176.71	47.12	15	6.8		1.41E-05	1.41E-05	1.94	62.36	7.24E+06		0.00015	1.00E-05	0.000009	indeterminate
1400	15	32.14577	176.71	47.12	15	7.9		1.41E-05	1.41E-05	1.94	62.36	8.45E+06		0.00015	1.00E-05	0.000008	indeterminate
1600	15	32.14577	176.71	47.12	15	9.1		1.41E-05	1.41E-05	1.94	62.36	9.65E+06		0.00015	1.00E-05	0.000007	indeterminate
1800	15	32.14577	176.71	47.12	15	10.2		1.41E-05	1.41E-05	1.94	62.36	1.09E+07		0.00015	1.00E-05	0.000006	indeterminate
2000	15	32.14577	176.71	47.12	15	11.3		1.41E-05	1.41E-05	1.94	62.36	1.21E+07		0.00015	1.00E-05	0.000005	indeterminate
2200	15	32.14577	176.71	47.12	15	12.4		1.41E-05	1.41E-05	1.94	62.36	1.33E+07		0.00015	1.00E-05	0.000005	indeterminate
2400	15	32.14577	176.71	47.12	15	13.6		1.41E-05	1.41E-05	1.94	62.36	1.45E+07		0.00015	1.00E-05	0.000004	indeterminate
2600	15	32.14577	176.71	47.12	15	14.7		1.41E-05	1.41E-05	1.94	62.36	1.57E+07		0.00015	1.00E-05	0.000004	indeterminate
2800	15	32.14577	176.71	47.12	15	15.8		1.41E-05	1.41E-05	1.94	62.36	1.69E+07		0.00015	1.00E-05	0.000004	indeterminate
3000	15	32.14577	176.71	47.12	15	17.0		1.41E-05	1.41E-05	1.94	62.36	1.81E+07		0.00015	1.00E-05	0.000004	indeterminate

HEAD LOSS CALCULATIONS - 4 OF 6



HEAD LOSS CALCULATIONS - 5 OF 6

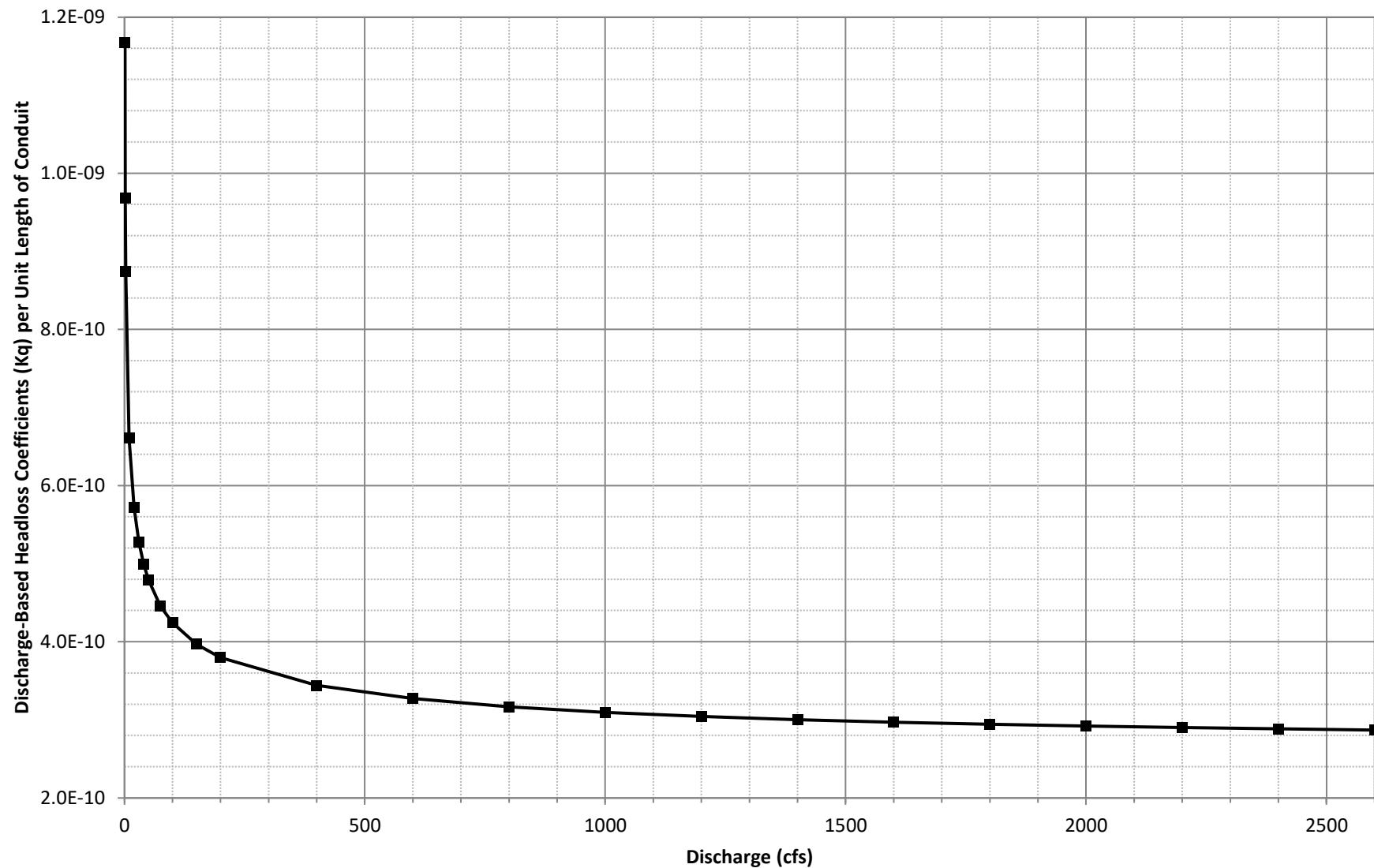


HEAD LOSS CALCULATIONS - 6 OF 6

Computed Discharge-Based Headloss Coefficients, K_q per Unit Length of Conduit

■ 15' Dia. Steel

Assumptions:
1) Water at 50°F
2) Gravity = 32.15 ft/s²
3) Roughness, k = 0.00015 feet (steel)



Pensacola Turbine-Generator Efficiency Hill Curve with Air Valves Closed

Discharge (cfs)	Total Turbine-Generator Efficiency (%) vs. Net Head (ft)												
	0	95	100	105	110	115	117.5	120	125	130	135	140	999
0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%
700	0.0%	60.6%	54.1%	47.3%	40.4%	32.9%	28.3%	23.9%	16.2%	12.9%	12.2%	11.6%	0.0%
750	0.0%	68.2%	63.5%	56.2%	50.6%	45.1%	42.4%	39.7%	34.6%	31.9%	31.3%	30.6%	0.0%
800	0.0%	72.6%	72.0%	66.4%	60.9%	55.4%	52.7%	50.2%	47.2%	46.0%	45.6%	45.2%	0.0%
850	0.0%	74.8%	75.1%	74.5%	71.8%	65.9%	63.2%	61.5%	58.5%	57.7%	57.4%	57.0%	0.0%
900	0.0%	76.5%	77.0%	77.0%	75.7%	73.9%	73.4%	72.5%	69.9%	69.6%	69.2%	68.9%	0.0%
950	0.0%	77.7%	78.4%	78.6%	77.9%	76.4%	76.0%	75.5%	74.8%	74.4%	74.3%	74.3%	0.0%
1000	0.0%	78.7%	79.6%	79.9%	79.4%	78.3%	77.8%	77.5%	76.6%	76.0%	75.7%	75.5%	0.0%
1050	0.0%	79.3%	80.4%	81.0%	80.6%	79.7%	79.4%	79.0%	78.1%	77.4%	77.0%	76.7%	0.0%
1100	0.0%	79.7%	80.8%	81.6%	81.5%	80.9%	80.6%	80.2%	79.5%	78.7%	78.3%	77.9%	0.0%
1150	0.0%	80.0%	81.2%	81.9%	82.2%	81.8%	81.6%	81.3%	80.6%	80.0%	79.5%	79.1%	0.0%
1200	0.0%	80.4%	81.4%	82.2%	82.9%	82.6%	82.3%	82.1%	81.7%	81.1%	80.6%	80.2%	0.0%
1250	0.0%	80.8%	81.6%	82.5%	83.3%	83.3%	83.1%	82.9%	82.5%	82.1%	81.7%	81.2%	0.0%
1300	0.0%	81.2%	81.9%	82.8%	83.6%	84.0%	83.8%	83.7%	83.4%	83.0%	82.6%	82.2%	0.0%
1350	0.0%	81.7%	82.4%	83.2%	83.9%	84.5%	84.4%	84.3%	84.1%	83.8%	83.5%	83.1%	0.0%
1400	0.0%	82.2%	82.9%	83.6%	84.2%	84.8%	85.0%	84.9%	84.8%	84.5%	84.3%	84.0%	0.0%
1450	0.0%	82.7%	83.5%	84.1%	84.6%	85.1%	85.3%	85.5%	85.5%	85.3%	85.0%	84.7%	0.0%
1500	0.0%	83.2%	84.0%	84.7%	85.2%	85.7%	85.9%	86.1%	86.2%	86.0%	85.7%	85.4%	0.0%
1550	0.0%	83.8%	84.6%	85.3%	85.8%	86.2%	86.5%	86.6%	86.8%	86.7%	86.4%	86.1%	0.0%
1600	0.0%	84.7%	85.4%	86.0%	86.5%	86.9%	87.0%	87.2%	87.4%	87.4%	87.1%	86.8%	0.0%
1650	0.0%	85.6%	86.2%	86.8%	87.2%	87.5%	87.6%	87.7%	88.0%	88.1%	87.8%	87.5%	0.0%
1700	0.0%	86.5%	87.0%	87.6%	88.0%	88.2%	88.3%	88.4%	88.7%	88.8%	88.6%	88.2%	0.0%
1750	0.0%	87.4%	88.0%	88.5%	88.9%	88.9%	89.0%	89.1%	89.4%	89.6%	89.4%	89.0%	0.0%
1800	0.0%	88.5%	88.9%	89.3%	89.6%	89.9%	89.9%	90.0%	90.1%	90.2%	90.2%	89.7%	0.0%
1850	0.0%	89.5%	89.7%	90.1%	90.4%	90.6%	90.7%	90.8%	90.8%	90.8%	90.8%	90.5%	0.0%
1900	0.0%	90.0%	90.5%	90.8%	91.1%	91.2%	91.3%	91.4%	91.4%	91.4%	91.3%	91.0%	0.0%
1950	0.0%	89.9%	90.7%	91.3%	91.5%	91.6%	91.7%	91.8%	91.9%	91.8%	91.8%	91.5%	0.0%
2000	0.0%	89.5%	90.0%	91.0%	91.3%	91.5%	91.6%	91.7%	91.9%	92.0%	91.8%	91.4%	0.0%
2050	0.0%	89.1%	89.5%	90.3%	90.9%	91.2%	91.3%	91.4%	91.5%	91.6%	91.7%	91.3%	0.0%
2100	0.0%	88.5%	89.1%	89.6%	90.4%	90.6%	90.8%	91.0%	91.1%	91.2%	91.3%	91.1%	0.0%
2150	0.0%	87.7%	88.7%	89.2%	89.8%	90.2%	90.3%	90.4%	90.7%	90.8%	90.8%	90.7%	0.0%
2200	0.0%	87.0%	88.0%	88.7%	89.3%	89.8%	89.9%	90.0%	90.1%	90.3%	90.4%	90.5%	0.0%
2250	0.0%	86.2%	87.2%	88.1%	88.8%	89.4%	89.6%	89.7%	89.8%	89.9%	90.0%	90.2%	0.0%
2300	0.0%	85.5%	86.5%	87.4%	88.2%	88.9%	89.1%	89.3%	89.4%	89.5%	89.7%	89.8%	0.0%
2350	0.0%	84.6%	85.7%	86.6%	87.5%	88.2%	88.5%	88.7%	89.0%	89.2%	89.2%	89.3%	0.0%
2400	0.0%	83.6%	84.8%	85.8%	86.7%	87.5%	87.8%	88.0%	88.4%	88.7%	88.8%	88.7%	0.0%
2450	0.0%	82.3%	83.7%	84.9%	85.9%	86.7%	87.0%	87.3%	87.7%	88.1%	88.3%	88.1%	0.0%
2500	0.0%	80.7%	82.5%	83.9%	84.9%	85.7%	86.0%	86.4%	86.9%	87.4%	87.6%	87.5%	0.0%
2550	0.0%	78.0%	80.9%	82.5%	83.7%	84.6%	85.0%	85.4%	86.0%	86.6%	86.8%	86.9%	0.0%
2600	0.0%	74.7%	78.1%	80.7%	82.3%	83.4%	83.9%	84.3%	85.0%	85.6%	85.9%	86.0%	0.0%

Pensacola Turbine-Generator Efficiency Hill Curve with Air Valves Fully Open (90 degrees open)

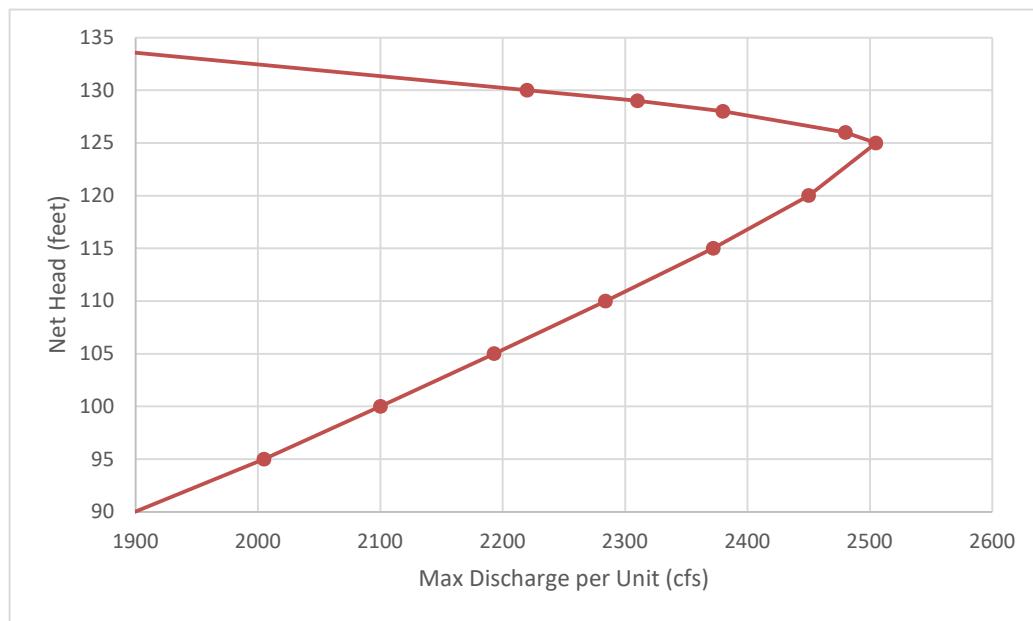
Discharge (cfs)	Total Turbine-Generator Efficiency (%) vs. Net Head (ft)												
	0	95	100	105	110	115	117.5	120	125	130	135	140	999
0	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%
700	0.0%	40.3%	36.0%	31.5%	26.9%	21.9%	18.9%	15.9%	10.8%	8.6%	8.1%	7.7%	0.0%
750	0.0%	38.0%	35.4%	31.3%	28.2%	25.1%	23.7%	22.2%	19.3%	17.8%	17.5%	17.1%	0.0%
800	0.0%	39.1%	38.8%	35.8%	32.8%	29.9%	28.4%	27.1%	25.5%	24.8%	24.6%	24.4%	0.0%
850	0.0%	39.4%	39.6%	39.2%	37.8%	34.7%	33.3%	32.4%	30.8%	30.4%	30.2%	30.0%	0.0%
900	0.0%	39.7%	40.0%	40.0%	39.3%	38.3%	38.1%	37.6%	36.3%	36.1%	35.9%	35.7%	0.0%
950	0.0%	44.0%	44.4%	44.4%	44.0%	43.2%	43.0%	42.7%	42.3%	42.1%	42.0%	42.0%	0.0%
1000	0.0%	48.4%	48.9%	49.1%	48.8%	48.1%	47.9%	47.6%	47.1%	46.7%	46.6%	46.4%	0.0%
1050	0.0%	50.7%	51.4%	51.8%	51.6%	51.0%	50.8%	50.6%	50.0%	49.6%	49.3%	49.1%	0.0%
1100	0.0%	53.1%	53.9%	54.4%	54.4%	54.0%	53.8%	53.5%	53.0%	52.6%	52.3%	52.0%	0.0%
1150	0.0%	55.6%	56.4%	56.9%	57.1%	56.9%	56.7%	56.6%	56.1%	55.6%	55.3%	55.0%	0.0%
1200	0.0%	58.2%	58.9%	59.5%	60.1%	59.9%	59.7%	59.5%	59.2%	58.8%	58.4%	58.1%	0.0%
1250	0.0%	60.9%	61.5%	62.2%	62.8%	62.8%	62.6%	62.5%	62.1%	61.8%	61.5%	61.2%	0.0%
1300	0.0%	63.5%	64.1%	64.8%	65.4%	65.7%	65.6%	65.5%	65.2%	64.9%	64.6%	64.3%	0.0%
1350	0.0%	66.4%	66.9%	67.5%	68.1%	68.6%	68.5%	68.4%	68.2%	68.0%	67.7%	67.4%	0.0%
1400	0.0%	69.2%	69.8%	70.3%	70.8%	71.3%	71.5%	71.4%	71.2%	71.1%	70.9%	70.6%	0.0%
1450	0.0%	72.2%	72.9%	73.4%	73.9%	74.3%	74.5%	74.6%	74.6%	74.4%	74.2%	74.0%	0.0%
1500	0.0%	75.1%	75.8%	76.4%	76.8%	77.3%	77.5%	77.7%	77.7%	77.6%	77.5%	77.2%	0.0%
1550	0.0%	77.7%	78.4%	79.1%	79.5%	79.9%	80.1%	80.3%	80.5%	80.4%	80.2%	79.9%	0.0%
1600	0.0%	79.7%	80.3%	80.9%	81.4%	81.7%	81.9%	82.1%	82.3%	82.2%	82.0%	81.8%	0.0%
1650	0.0%	81.3%	81.9%	82.4%	82.8%	83.1%	83.2%	83.3%	83.6%	83.7%	83.4%	83.2%	0.0%
1700	0.0%	82.2%	82.7%	83.3%	83.7%	83.8%	83.9%	84.1%	84.4%	84.5%	84.3%	83.9%	0.0%
1750	0.0%	82.7%	83.3%	83.8%	84.3%	84.3%	84.3%	84.5%	84.8%	84.9%	84.7%	84.4%	0.0%
1800	0.0%	83.3%	83.8%	84.2%	84.5%	84.7%	84.8%	84.9%	85.0%	85.1%	85.1%	84.7%	0.0%
1850	0.0%	84.0%	84.3%	84.7%	85.0%	85.1%	85.3%	85.4%	85.3%	85.4%	85.4%	85.0%	0.0%
1900	0.0%	84.3%	84.8%	85.1%	85.4%	85.5%	85.6%	85.7%	85.7%	85.7%	85.6%	85.3%	0.0%
1950	0.0%	83.9%	84.6%	85.2%	85.3%	85.5%	85.6%	85.7%	85.7%	85.7%	85.7%	85.5%	0.0%
2000	0.0%	83.2%	83.7%	84.6%	85.0%	85.1%	85.2%	85.3%	85.5%	85.6%	85.5%	85.2%	0.0%
2050	0.0%	82.3%	82.7%	83.4%	84.0%	84.3%	84.4%	84.5%	84.6%	84.8%	84.9%	84.6%	0.0%
2100	0.0%	81.3%	81.9%	82.4%	83.1%	83.4%	83.5%	83.6%	83.8%	83.9%	84.1%	83.9%	0.0%
2150	0.0%	80.1%	81.0%	81.5%	82.1%	82.4%	82.5%	82.6%	82.9%	83.1%	83.2%	83.1%	0.0%
2200	0.0%	78.6%	79.5%	80.2%	80.8%	81.2%	81.3%	81.4%	81.6%	81.8%	82.0%	82.1%	0.0%
2250	0.0%	77.1%	78.0%	78.8%	79.5%	80.0%	80.1%	80.3%	80.5%	80.7%	80.9%	81.0%	0.0%
2300	0.0%	75.7%	76.6%	77.4%	78.1%	78.8%	79.0%	79.2%	79.4%	79.6%	79.8%	79.9%	0.0%
2350	0.0%	74.5%	75.5%	76.3%	77.1%	77.7%	78.0%	78.3%	78.6%	78.8%	78.9%	79.1%	0.0%
2400	0.0%	73.3%	74.3%	75.2%	76.0%	76.8%	77.1%	77.3%	77.8%	78.1%	78.2%	78.3%	0.0%
2450	0.0%	72.0%	73.2%	74.2%	75.1%	75.8%	76.2%	76.5%	77.0%	77.3%	77.6%	77.6%	0.0%
2500	0.0%	70.7%	72.3%	73.5%	74.4%	75.1%	75.5%	75.8%	76.5%	76.9%	77.2%	77.3%	0.0%
2550	0.0%	68.5%	71.1%	72.5%	73.6%	74.4%	74.8%	75.2%	75.9%	76.4%	76.8%	77.0%	0.0%
2600	0.0%	66.0%	69.0%	71.2%	72.6%	73.7%	74.2%	74.6%	75.3%	75.9%	76.3%	76.7%	0.0%

Kerr Turbine Efficiency Hill Curve (Refurbished Units 2010)

Discharge (cfs)	Total Turbine-Generator Efficiency (%) vs. Head (ft)								
	0	35	51	54	56	58	61	80	999
0	0%	61.2%	76.5%	76.5%	76.7%	76.9%	76.9%	61.5%	0%
2000	0%	65.6%	83.8%	84.1%	84.4%	84.7%	84.7%	67.9%	0%
2500	0%	66.8%	85.3%	85.5%	85.8%	86.1%	86.2%	69.0%	0%
3000	0%	68.0%	86.7%	86.9%	87.2%	87.6%	87.7%	70.3%	0%
3500	0%	69.2%	88.1%	88.3%	88.7%	89.1%	89.1%	71.3%	0%
4000	0%	70.2%	89.4%	89.6%	89.9%	90.2%	90.1%	72.2%	0%
4500	0%	70.7%	90.0%	90.1%	90.4%	90.7%	90.7%	72.6%	0%
4750	0%	71.2%	90.5%	90.6%	90.9%	91.2%	91.2%	73.0%	0%
5000	0%	71.6%	90.9%	91.1%	91.3%	91.6%	91.6%	73.3%	0%
5250	0%	71.9%	91.3%	91.4%	91.7%	92.0%	91.9%	73.5%	0%
5500	0%	72.1%	91.4%	91.6%	91.9%	92.1%	92.0%	73.6%	0%
6000	0%	72.2%	91.4%	91.5%	91.7%	92.0%	91.9%	73.5%	0%
6500	0%	72.1%	91.0%	91.1%	91.3%	91.6%	91.5%	73.2%	0%
7000	0%	71.6%	90.2%	90.3%	90.6%	90.8%	90.7%	72.6%	0%
7500	0%	71.5%	89.8%	89.9%	90.2%	90.4%	90.3%	72.2%	0%
8500	0%	70.1%	87.9%	88.0%	88.2%	88.4%	88.2%	70.5%	0%
9000	0%	69.3%	87.0%	87.0%	87.2%	87.3%	87.2%	69.6%	0%

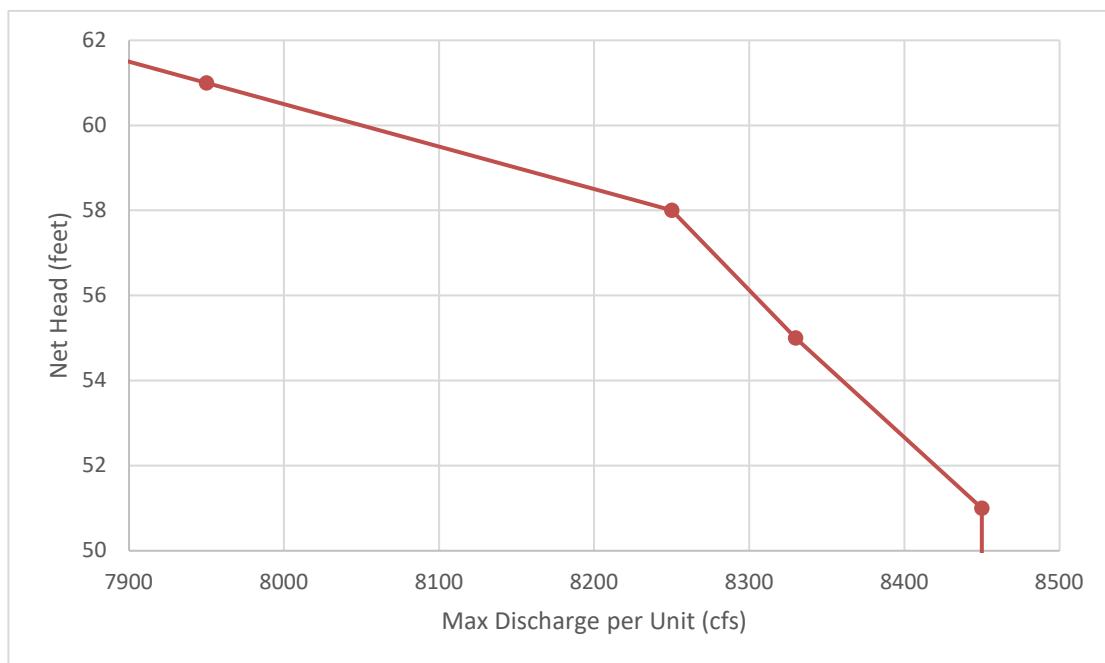
Pensacola Dam Maximum Discharge (cfs) per Unit vs. Net HeadDischarge Limited by Cavitation - 6 Units Total

Net Head (ft)	Max Discharge per Unit (cfs)	Max Discharge Total (cfs)
0	0	0
95	2005	12030
100	2100	12600
105	2193	13158
110	2284	13704
115	2372	14232
120	2450	14700
125	2505	15030
126	2480	14880
128	2380	14280
129	2310	13860
130	2220	13320
154.67	0	0
999.00	0	0



Kerr Dam Maximum Discharge (cfs) per Unit vs. Net HeadDischarge Limited by Cavitation - 4 Units Total

Net Head (ft)	Max Discharge per Unit (cfs)	Max Discharge Total (cfs)
0	8450	33800
51	8450	33800
55	8330	33320
58	8250	33000
61	7950	31800
140.50	0	0
999	0	0

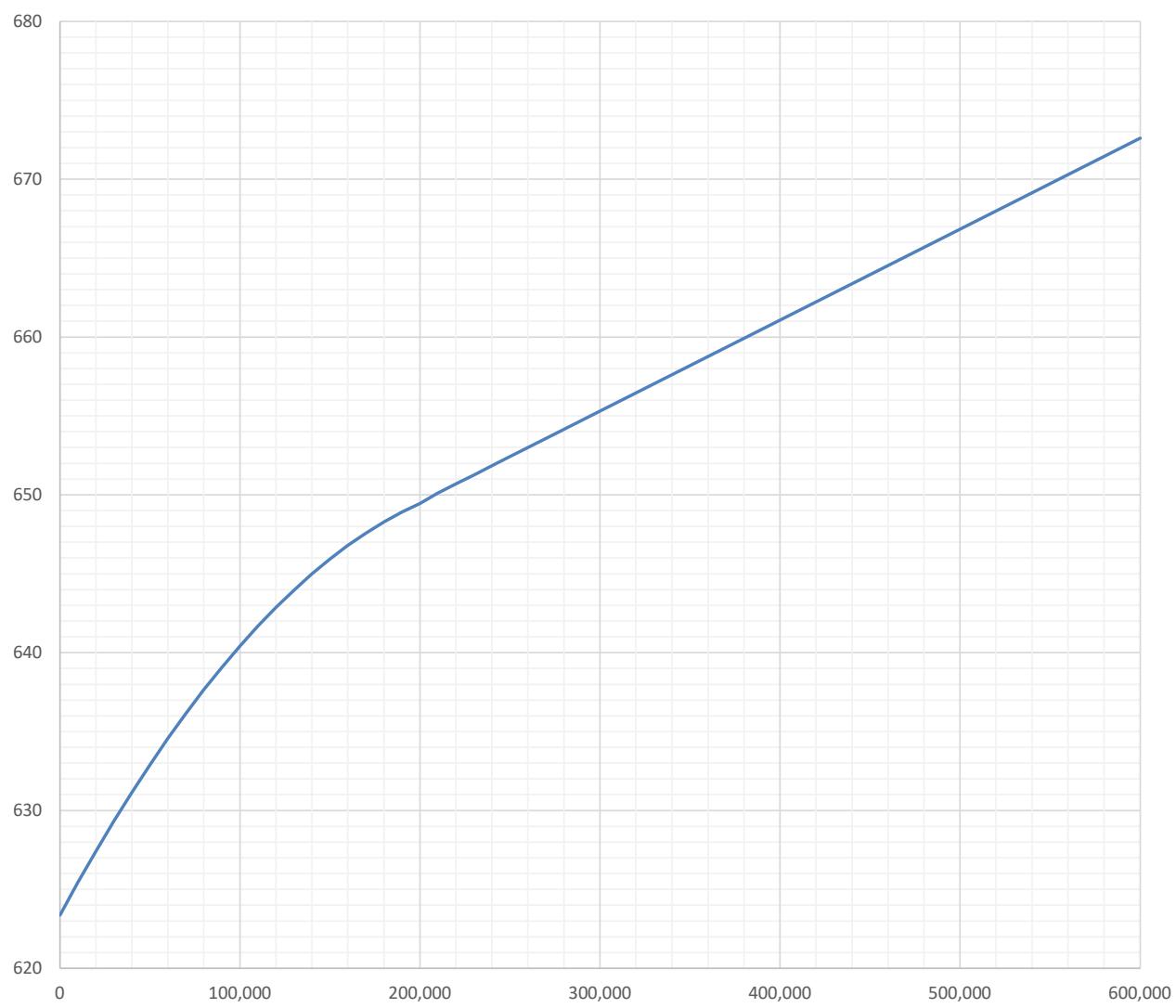


Pensacola Dam Tailwater Rating Curve

Discharge TW Elev
(cfs) (ft PD)

0	623.37
10000	625.43
20000	627.42
30000	629.32
40000	631.15
50000	632.89
60000	634.56
70000	636.14
80000	637.65
90000	639.07
100000	640.41
110000	641.68
120000	642.86
130000	643.97
140000	644.99
150000	645.93
160000	646.80
170000	647.58
180000	648.28
190000	648.90
200000	649.45
210000	650.11
220000	650.69
230000	651.27
240000	651.84
250000	652.42
275000	653.86
300000	655.30
600000	672.61

Assumed Pensacola Dam Tailwater Rating Curve



Pensacola Dissolved Oxygen Air Valve Data

